

# Converting Four lane roadways into Five lane roadways on Urban Structure: Study on Safety Effectiveness



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## Abstract

- In urban or suburban areas with large number of access points, four-lane undivided highways are prone to crashes due to left-turning and through movements in a single lane.
- Expensive countermeasures like conversion from undivided to divided road are recommended by many studies. **One inexpensive alternative is reconfiguring the existing roadways by either increasing or decreasing the number of lanes.**
- This study investigated the safety impact of **converting four lane undivided roadways (4U) to five lane undivided roadways (5T) with a two-way left-turn lane (TWLTL)**. This study used Empirical Bayes method to determine the safety impact of this inexpensive countermeasure.
- The findings of the current study indicated a positive safety impact. The benefit-cost ratio of this conversion ranges from 97 to 379. The current findings indicate that conversion of 4U to 5T is a feasible inexpensive solution for urban structure.

## Methodology

- This study selected eight sites from Louisiana. This countermeasure requires adding of a TWLTL by restriping (see **Figure 1**).
- The observational before-after method used in this study is empirical Bayes (EB) Method.
- This method accounts for the effect of regression-to-the-mean, changes in traffic volume (**Table 1**), and other potential changes in the roadway features during the before and after time periods.
- In accounting for regression-to-the-mean, the number of crashes expected in the before period without the treatment ( $N_{predicted, t, b}$ ) is a weighted average of information from two sources:
  - The number of crashes observed in the before period at the treated sites ( $N_{observed, t, b}$ ).
  - The number of crashes predicted at the treated sites based on reference sites with similar traffic and physical characteristics ( $N_{predicted, t, b}$ ).

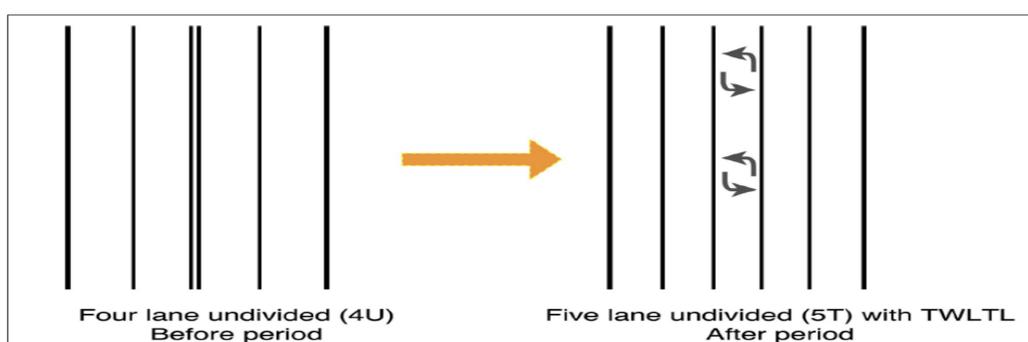


Figure 1. 4U to 5T Conversion

Table 1. AADT and Overserved Crashes in Before-After Years

| Sites  | Before Period |            |               | After Period |            |               |
|--------|---------------|------------|---------------|--------------|------------|---------------|
|        | Years         | AADT (vpd) | Total Crashes | Years        | AADT (vpd) | Total Crashes |
| Site 1 | 2004-2006     | 19,867     | 21            | 2008-2010    | 19,767     | 9             |
| Site 2 | 2004-2006     | 19,867     | 118           | 2008-2010    | 19,767     | 47            |
| Site 3 | 2008-2010     | 6,833      | 39            | 2012-2014    | 7,900      | 20            |
| Site 4 | 2008-2010     | 19,200     | 126           | 2012-2014    | 21,000     | 114           |
| Site 5 | 2000-2002     | 23,888     | 358           | 2004-2006    | 26,580     | 148           |
| Site 6 | 2004-2006     | 21,367     | 65            | 2008-2010    | 21,100     | 51            |
| Site 7 | 2002-2004     | 27,467     | 116           | 2006-2008    | 27,000     | 75            |
| Site 8 | 1996-1998     | 14,693     | 115           | 2000-2002    | 14,967     | 79            |

## Safety Effectiveness

- The EB estimate of the expected number of crashes without treatment,  $N_{expected, t, b}$ , is computed from the following equation:
 
$$N_{expected, t, b} = w \times N_{predicted, t, b} + (1 - w) \times N_{observed, t, b}$$

$$w = \frac{1}{1 + k \times \sum_{all\ study\ years} N_{predicted}}$$
 where,
  - w = weighted adjustment to be placed on the predictive model estimate; and
  - k = over-dispersion parameter of the associated SPF used to estimate  $N_{predicted}$ .
- Table 2** enlists the values of site specific Crash Modification Factor (CMF), standard deviations, and 95% confidence interval (CI).
- The **CMF values range from 0.35 to 0.84** (except site 6; in which CMF is greater than 1).

Table 2. CMF values and their Variances

| Sites  | CMF  | Var(CMF) | sd(CMF) | 95% CI of CMF |
|--------|------|----------|---------|---------------|
| Site 1 | 0.48 | 0.03     | 0.18    | (0.13, 0.84)  |
| Site 2 | 0.42 | 0.01     | 0.07    | (0.28, 0.56)  |
| Site 3 | 0.62 | 0.03     | 0.17    | (0.3, 0.95)   |
| Site 4 | 0.84 | 0.01     | 0.11    | (0.63, 1.05)  |
| Site 5 | 0.35 | 0.00     | 0.03    | (0.28, 0.42)  |
| Site 6 | 1.18 | 0.05     | 0.22    | (0.76, 1.6)   |
| Site 7 | 0.65 | 0.01     | 0.10    | (0.47, 0.84)  |
| Site 8 | 0.64 | 0.01     | 0.09    | (0.46, 0.82)  |

## Benefit-Cost Analysis

The benefit cost ratio for the treatment sites range from 97 to 379. The benefit-cost ratio for all eight segments is shown in **Table 3**.

Table 3. Benefit-Cost Ratios

| Site                   | Total Benefits (\$) | Total Cost (\$) | B/C Ratio |
|------------------------|---------------------|-----------------|-----------|
| Site 1                 | 278,951             | 2,863           | 97        |
| Site 2                 | 1,387,818           | 4,809           | 289       |
| Site 3                 | 810,675             | 5,382           | 151       |
| Site 4                 | 1,142,767           | 7,672           | 149       |
| Site 5                 | 3,039,771           | 14,084          | 216       |
| Site 6                 | 651,252             | 1,718           | 379       |
| Site 7                 | 630,598             | 4,580           | 138       |
| Site 8                 | 1,076,223           | 9,046           | 119       |
| PDO crash cost (\$)    | 6,623               |                 |           |
| Injury Crash cost (\$) | 46,518              |                 |           |
| Cost per mile (\$)     | 11,450              |                 |           |

## Conclusion

- This study suggests that inserting a two way left turn lane on four lane undivided urban highways can have significant benefit.
- It is also important to note that one-size-fits-all solutions do not usually work in highway safety issues. Caution must be taken when applying this crash countermeasure in other locations.

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## References

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